Technical Assignment II

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Executive Summary

This technical assignment analyzes the key construction management aspects of The Thames St. Wharf Office Building. Included in this report is a detailed project schedule, a site plan for each phase of construction, a detailed structural estimate and a general conditions estimate. The detailed structural estimate is compared to the actual construction costs of the project. A review of some critical industries that were discussed at the 2009 Pace Roundtable is also included in this report.

The project schedule developed in this assignment contains a lot more detail and goes further in depth that the schedule in the previous technical assignment. It shows the major phases of construction and breaks down the major activities in each phase of construction. The major phases for the Thames St. Wharf Office Building are sitework, superstructure, enclosure and interior finishes. Each Phase is broken into multiple sub-phases and each sub-phase is broken into activities by floor. A schedule showing the phases with their corresponding sub-phases is shown as is a complete detailed schedule. Both schedules show the project being completed under ideal conditions and circumstances.

A site plan was developed for each of the four major phase of construction. The ample amount of space available on the site made site plans simple and straight forward to develop. Each site plan illustrates key equipment and space necessary to successfully complete its respective phase.

A detailed structural estimate was compiled including the costs for the foundation, post-tensioned structural concrete system and the structural steel penthouse. A detailed take-off was completed for each system and values from RS Means 2009 were used to determine the total cost. The cost for the entire concrete structural system, including the foundation, is \$8,120,710. This estimate value is less than 1% of the actual construction cost for the concrete. The structural steel estimated cost is \$782,347. This compares nicely with the total contract value of \$1.1 Million for all of the metals used in the project.

The general conditions costs for The Thames St. Wharf Office Building were developed using RS Means 2009 and the general conditions costs in Justin Wingenfield's Tech II report. The total value for the general conditions costs was \$1,746,410 over the life of the project. This breaks down to a value of \$11,856 per week.

The final topic discussed in this report is a summary of the 2009 PACE Roundtable Conference. It includes a general overview of what took place and what was talked about at the conference, an indepth look at the breakout session attended, an analysis of how the topics covered could be applied to The Thames St. Wharf Office Building, and any key industry contacts made.

Detailed Project Schedule

There detailed construction schedule was developed using four major phases of construction. The phases are sitework, superstructure, enclosure and interior finishes. The phases overlap slightly as show on the schedule. Listed below in table 1.1 is an abridged version of the schedule, for the complete detailed schedule please view **Appendix A.** It is important to note that MEP activities take place under each phase of construction except the enclosure phase.

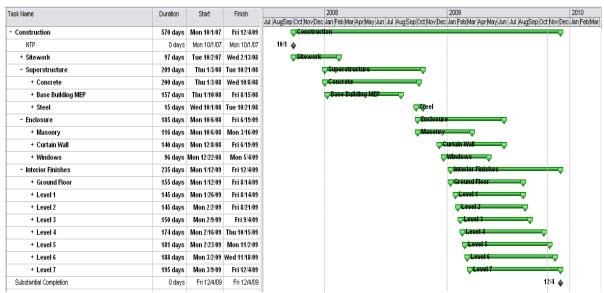


Table 1.1 – Abridged Detailed Project Schedule

The sitework phase is broken up by activity and includes demolition of the existing parking lot because it is a small activity. The superstructure and interior finishes phases are broken up by floor and the enclosure phase is broken up by each face of the building. The superstructure phase includes the concrete pile caps and slab on grade because they are done by the same subcontractor as the rest of the concrete structural system. It is important to note that on the actual finish schedule the Ground Level – Level 3 are broken into two sub-categories, floor wide and core, but they were condensed on this schedule to limit the number of activities. Levels 4 – 7 are broken into four sub-categories; floor wide, core, tenant fit out and central command/ IDF room. Again floor wide and core have been condensed into one sub-category.

For the finish phase work began on the ground floor and moved up for everything except for framing and finishing which started on the Level 1 and moved vertically and finished on the Ground Level. This is what causes the gap between "Frame, Finish & Paint — Core" and "Duct Layout" to increase on each floor. The reason this sequence was chosen is currently unknown.

This schedule illustrates ideal conditions for the project. Unfortunately delays have occurred so far in the project. The actual extent and reason for the delays is unknown due to the inability to contact the original construction manager. All that is currently known is that there was a delay with the backfilling and the underground utilities that caused the concrete contractor to mobilize, demobilize for six weeks and then remobilize again. What caused the delay in backfilling and the underground utilities

is unknown. There were also issues with the concrete construction that also caused delays; the extent and reason are also unknown. Work sequencing was also disrupted slightly during the switch from SBER construction to Armada Hoffler Construction. Some work was paused as contracts were finalized and agreements were reached. The sequence disruption cost approximately two weeks of delay but there is hope that the lost time can be made up.

Site Layout Planning

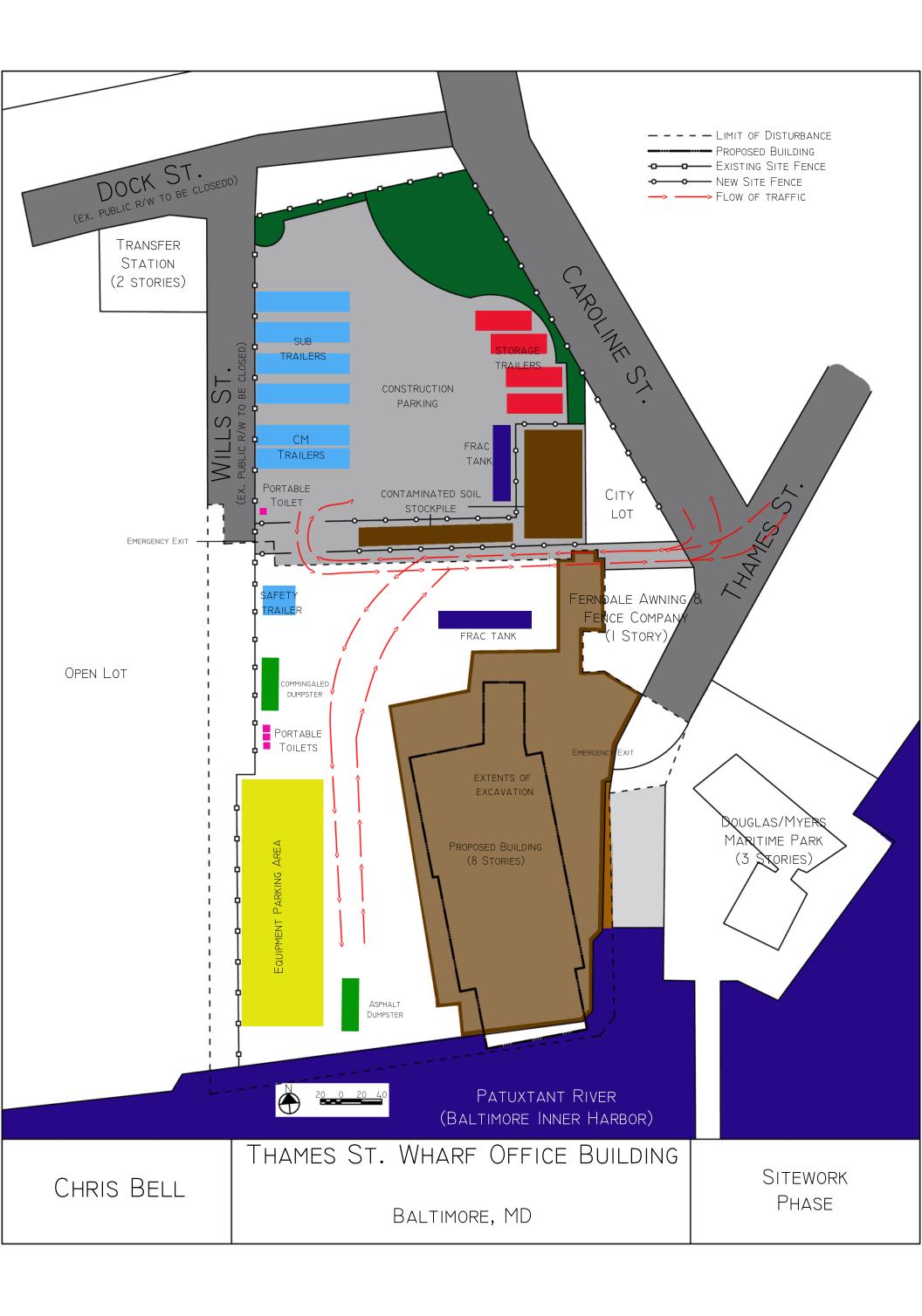
No actual site plans were available for reference so a site plan has been created for each phase of construction based on what knowledge was available and what would create the least cluttered site. Fortunately the site is very large and already separated into a northern and a southern portion by Block St. This allows for the site trailers, storage sheds and construction parking to be located away from all construction activities and therefore safe. The northern half of the site plan does not change between each phase of construction. The site fencing and on-site traffic patterns do not change between phases either.

Sitework:

The sitework phase of construction was the shortest phase of construction and had the least number of people involved with it. The entire parking lot in the southern portion of the site did not need to be removed for construction to be successfully completed so it was only demolished in areas that needed to be excavated for underground utilities and pile driving. The soil that was excavated was moved into a segregated section in the northern half of the site plan and separated into two piles. The first pile was for the soils that contained the most contamination and the other pile was for soils that only had a limited amount. The cut off point for each contamination level is not known. The more contaminated soil was to be stored on-site until a disposal facility could be found. The soil with the lower contamination level was also to be stored on-site and used as controlled backfill later in the sitework phase. The remaining low contamination soil was then to be disposed of with the high contamination soil. Frac tanks were placed at two locations on site to help gather contaminated particles after a rain and would be removed as they were deemed no longer necessary. No excavation support was needed soil cohesiveness allowed for a step back of 2:1

A separate asphalt dumpster was utilized because the asphalt also had traces of volatile compounds and needed to be disposed of properly. Additionally a commingled dumpster was placed on-site for the remainder of the refuse created. Commingled dumpsters were used to make recycling easier and lower the number of dumpsters needed on-site at one time. If separate dumpsters were used for each material at least four dumpsters would be needed on-site at any one time, at one for each material.

The following page illustrates the site plan for the sitework phase.



Superstructure:

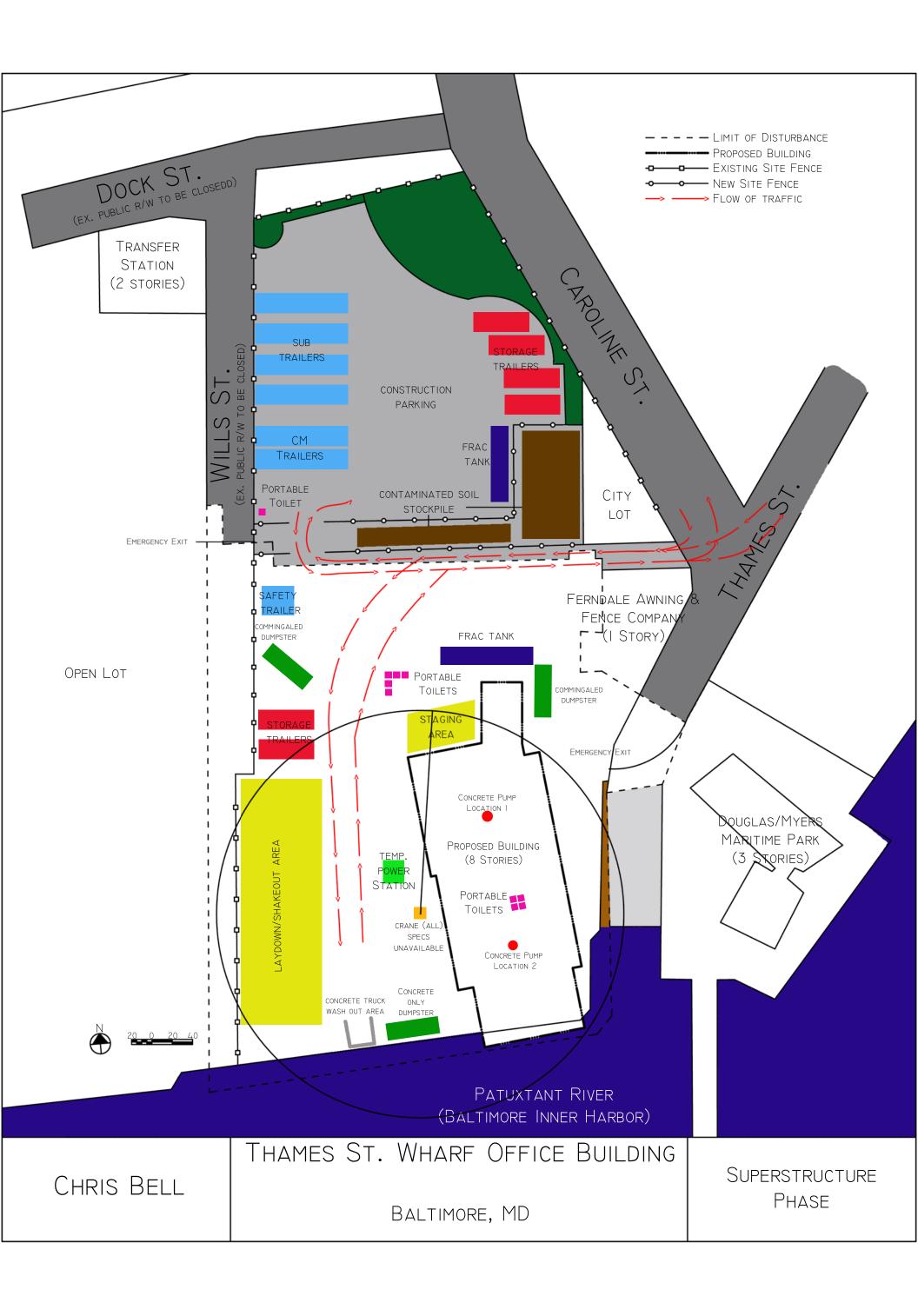
The superstructure phase was the only phase of the project that utilized a permanent tower crane. It was placed outside the building limits because there was available space and it was easier to erect and dismantle. The exact specifications of the crane are not known but it is known that it covered the entire floor area for L1-PH. The only area of the building that was not under the reach of the tower crane was northern extension on the ground floor. To place the rebar cages for the columns in that area a boom lift was used and concrete was pumped into the formwork.

The rest of the columns in the building were placed using crane and bucket. The floor slabs and shear walls were all placed by pumping. There were two pump locations in the building footprint. Pump Location 1 was used to place the northern $2/3^{rds}$ of the slabs and the northern shear walls. Pump Location 2 was used for the southern $1/3^{rd}$ and for the southern shear walls. All reinforcing deliveries were made to the lay down/shakeout area to the west of the building and the days picks would be organized in the staging area to the north of the building.

For the steel construction in the PH the steel contractor brought in their own crawler crane. It was parked in the same location as the tower crane. Again the crane specifications are unknown.

Other notable changes in the site plan are the addition of a dedicated concrete dumpster and another commingled dumpster. A wash out area for the concrete trucks was placed next to concrete dumpster. More personnel are expected on-site during this phase so additional portable toilets are on-site, including some inside the building limits that are lifted from floor-to-floor as work progresses vertically. A temporary power station was also established during this phase. It powers everything needed for construction including the crane. The exact specs of the temporary power system are unknown. Additional material storage trailers were added on the southern half of the site for the mechanical and electrical subcontractors. They were placed in the southern half to limit the distance those subs needed to transport their materials. The northern storage trailers were used mostly for equipment storage.

The following site plan illustrates the superstructure phase with the tower crane. The crawler crane was omitted in favor of clarity on the drawing.

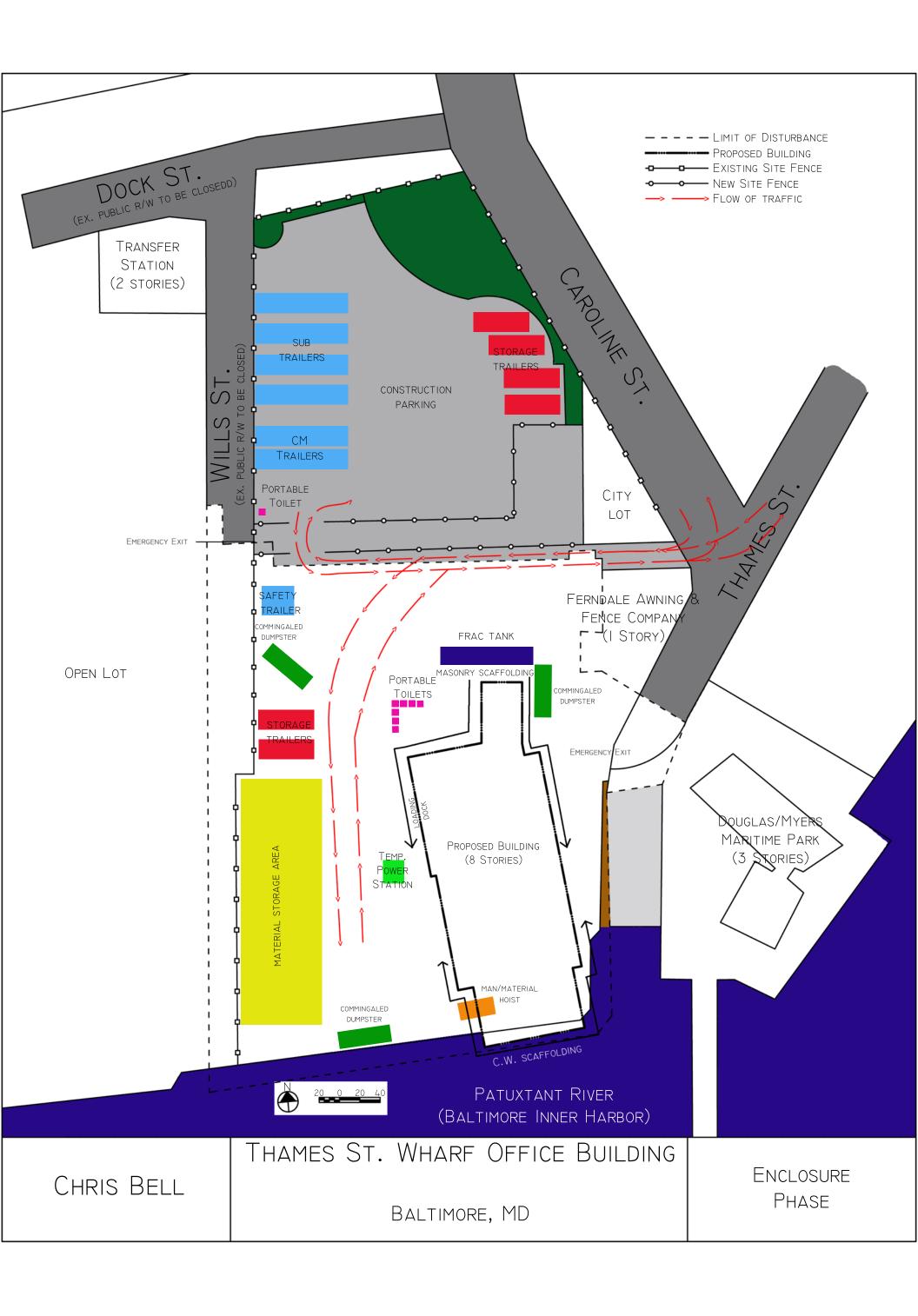


Enclosure:

The enclosure phase of the project is the phase with the least amount of information available. Scaffolding was used to for both the brickwork and the curtain wall installation. Based on the limited schedule that is available for this phase the assumed work progression is from north-to-south for the masonry, starting on the North and East faces of the building and the curtain wall started in the south and worked north. The curtain wall was stick built and the pieces were light enough to be lifted by one or two people so no cranes were needed. The windows were installed from the inside so no scaffolding was needed.

The tower crane is no longer needed for construction and has been replaced with a man and materials elevator on the south-western corner of the building. There is also a loading dock on the western face of the building in the northern half. During the enclosure phase the soil stockpiles were removed reducing the need for two frac tanks on-site. One was kept just as a safety precaution in case there were severe weather conditions that dictated a need for one. The concrete wash out area was removed and the concrete only dumpster was replaced with a commingled dumpster. The lay down/shakeout area is now a material storage area during this phase.

The following site plan illustrates the enclosure phase.



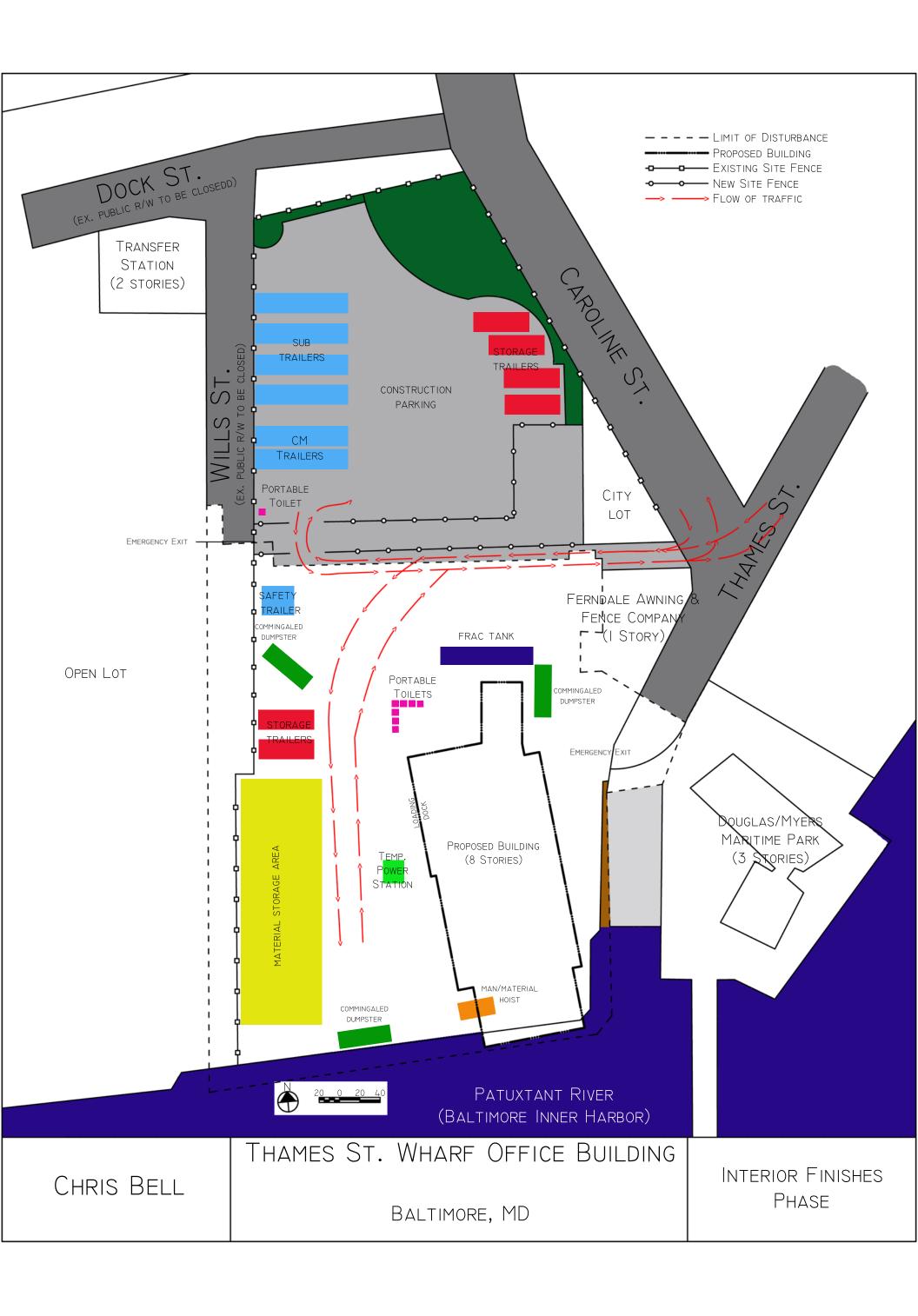
Interior Finish:

The interior finish phase is the final phase of construction. There are very few changes on the site plan from the enclosure phase. These two phases overlap a significant amount giving reason to the similarities in the site plans. The only major difference is the absence of scaffolding.

All materials that need be stored on-site and protected from the weather are stored inside of the building. There is a lot of empty space inside the building because the floors from the Ground Level through Level 3 are being built as core and shell.

To place the roof mechanical equipment the mechanical subcontractor brought in a crawler crane as well. It was also placed in the same place as the other two cranes had been previously; it was left out of this site plan for clarity. Again the specs for the crane are unknown. To move and install the SCU's on each floor a fork lift was used. The mechanical subcontractor was responsible for the forklift.

The following site plan illustrates the interior finish phase.



Detailed Structural Systems Estimate

The Thames St. Wharf Office Building consists of a reinforced concrete on metal piles foundation, a post-tensioned concrete superstructure and a structural steel penthouse. The estimates for the foundation and structural steel were created using a complete takeoff of both systems. For the post-tensioned concrete superstructure a take-off was done for Level 4 and used as the average for the other levels. All of the unit prices for the estimate come from RS Means Online using the 2009 cost book with the location factor for Baltimore, MD included. Table 3.1 - Table 3.3 show the estimate results. For an overview of the estimate take off quantities please refer to **Appendix B**. The complete take off is not provided due to its length and size.

Material Costs							
Item	Quantity Total Cost						
Concrete	13144 CY	\$ 1,597,868					
Mild Steel	13.28 Tons	\$ 2,059,134					
PT Tendons	38129 lbs	\$ 77,784					
Formwork	403765 SF	\$ 1,534,610					
Piles	19769 VLF	\$ 655,611					
Structural Steel		\$ 737,758.32					
	Total	\$ 6,662,765					

Table 3.1

Detailed Concrete Estimate						
			Piles			
Item	Quantity	Unit	Material Cost	Labor Cost	Eqpt Cost	Total Cost
14" Piles - Concrete Pile	15839.21		509705.78			\$ 679,502
16" Piles - Concrete Pile	3929.58	V.L.F	145905.12	22948.72		
					Subtotal	\$ 869,969
		C	oncrete			
Item	Quantity	Unit	Material Cost	Labor Cost	Eqpt Cost	Total Cost
4000 psi concrete - pumped	1455.54	CY	154287.58	24307.57	8878.81	\$ 187,474
5000 psi concrete - pumped 5000psi concrete - crane and	9096.82	CY	1009746.74	288823.95	65724.51	\$ 1,364,295
bucket 5000 psi concrete - direct	597.33	CY	66304.00	20906.67	4241.07	\$ 91,452
chute	456.30	CY	50649.25	8053.69	264.65	\$ 58,968
8000 psi concrete - pumped	1538.25	CY	316880.01	30380.49	11075.42	· ·
					Subtotal	\$ 2,060,524
		PT	Tendons			
ltem	Quantity	Unit	Material Cost	Labor Cost	Eqpt Cost	Total Cost
PT Tendons	0.00	lb	0.00	0.00	0.00	\$ -
					Subtotal	\$ -
		Mild	Reinforcing			•
Item	Quantity	Unit	Material Cost	Labor Cost	Eqpt Cost	Total Cost
#3-#7	706.90	Ton	1087728.41	284993.85	0.00	\$ 1,372,722
#8-#18	621.12	Ton	971405.33	180405.18	0.00	\$ 1,151,811
Sorting	1328.02	Ton	0.00	16919.01	10996.03	\$ 27,915
Crane	1328.02	Ton	0.00	18419.68	11965.49	\$ 30,385
					Subtotal	\$ 2,582,833
		Fo	ormwork			
Item	Quantity	Unit	Material Cost	Labor Cost	Eqpt Cost	Total Cost
Slab Formwork	214641.46	SF	989497.11	474357.62	0.00	\$ 1,463,855
Beam Formwork	179196.05	SF	510708.74	582387.16	0.00	\$ 1,093,096
Column & SW	6858.44	SF	24244.60	9601.82	411.51	\$ 34,258
Grade beams	3069.33	SF	10159.49	6015.89	0.00	\$ 16,175
					Subtotal	\$ 2,607,384
					Total	\$ 8,120,710

Table 3.2

	Detailed Steel Estimate										
	Beams										
		Length[
Туре	Size	ft]	Quantity	Ma	aterial Cost	Lal	bor Cost	Eq	ot. Cost	То	tal Cost
W	18X35	36	9	\$	17,641.80	\$	923.40	\$	1,075.68	\$	19,640.88
W	10x49	36	3	\$	8,739.36	\$	307.80	\$	358.56	\$	9,405.72
W	18x40	30	3	\$	6,092.10	\$	256.50	\$	298.80	\$	6,647.40
W	18x40	24	4	\$	6,498.24	\$	273.60	\$	318.72	\$	7,090.56
W	18x40	38		\$	2,572.22	\$	108.30	\$	126.16	\$	2,806.68
W	10x12	40	2	\$	1,582.40	\$	208.80	\$	243.20	\$	2,034.40
W	21x44	40		\$	14,486.00	\$	440.00	\$	368.00	\$	15,294.00
W	12X48	40	3	\$	9,890.40	\$	250.80	\$	291.60	\$	10,432.80
W	24X55	30	2	\$	5,454.60	\$	127.20	\$	106.20	\$	5,688.00
W	21X44	30	3	\$	6,518.70	\$	198.00	\$	165.60	\$	6,882.30
W	12X14	7.25	4	\$	767.63	\$	51.62	\$	60.32	\$	879.57
W	16X26	12	2	\$	1,031.04	\$	37.68	\$	43.68	\$	1,112.40
W	16X26	24	2	\$	2,062.08	\$	75.36	\$	87.36	\$	2,224.80
W	12X38	12	1	\$	695.28	\$	23.16	\$	27.00	\$	745.44
HSS	16X12X5/8	45	3	\$	235,665.45	\$	4,660.20	\$	5,425.65	\$	245,751.30
HSS	16X12X5/8	38	2	\$	132,670.92	\$	2,623.52	\$	3,054.44	\$	138,348.88
С	6X10.5	45	6	\$	54,621.00	\$	3,916.35	\$	4,527.90	\$	63,065.25
								Sul	ototal	\$	538,050.38
				(Columns						
		Length[
Туре	Size	ft]	Quantity		aterial Cost		bor Cost		ot. Cost		tal Cost
W	10X39	16.5		\$	11,052.86	\$	225.72	\$	262.85	\$	11,278.58
HSS	6X6X1/2	16.5	14		93,462.60		6,701.31	\$	7,747.74	-	100,163.91
HSS	12X12X1/4	16.5	3	\$	80,357.31	\$	1,627.56	\$	1,893.38	\$	81,984.87
								Sul	ototal	\$	193,427.36
				Cro	oss Bracing						
		Length[
Туре	Size	ft]	Quantity		aterial Cost		bor Cost	Eq	ot. Cost	То	tal Cost
L	4x4x1/4	39.5	2	\$	9,786.13	I '	1,069.66		1,241.88	\$	10,855.79
L	4x4x1/4	43	2	\$	10,653.25	\$	1,164.44	\$	1,351.92	\$	11,817.69
								Sul	ototal	\$	22,673.48
					Decking						
		Length[
Туре	Size	ft]	Quantity	Ma	aterial Cost	Lal	bor Cost	Eq	ot. Cost	То	tal Cost
20G											
Galvanized	3"	106	76	\$	25,456.96	\$	2,416.80	\$	322.24	\$	28,196.00
								Sul	ototal	\$	28,196.00
								Tof	tal	\$	782,347.21

For the concrete estimate a major assumption was that all of the columns in the building were 24"x24"X12' with 5000 psi concrete. The majority of the columns in the building are 24"x24"X12' with 5000 psi concrete so this assumption is not far off. It was also assumed that each pile was driven a depth of 64.25 ft. This value was listed in the foundation drawings as the average

Overall the concrete estimate that was developed is off of the actual cost by less than 1%. The actual concrete cost was \$8.2 Million. The difference is most likely due to the general conditions costs for the concrete subcontractor and the assumptions that were made in the estimate. A list of assumptions is listed below.

Concrete Estimate Assumptions:

- 1. The cost of each pile includes concrete being placed inside but does not include reinforcing
- 2. No reinforcing is epoxy coated
- 3. Each PT beam contains three (3) tendon bundles containing (4) tendons
- 4. Crane cost is included in equipment unit costs
- 5. Open-shop labor

The steel value also seems close to the actual construction cost. The exact percentage difference is unknown because the structural steel cost that has been reported is combined with the miscellaneous metal cost. The total misc. metal cost is \$1.1 Million. It stands to reason that the majority of the misc. metal cost is for the structural steel making the estimate provided pretty accurate.

General Conditions Estimate

The general conditions costs for Thames St. Wharf Office Building were not available as a reference so this estimate is based on RS Means values and the general conditions costs listed in Justin Wingenfield's Tech II assignment. His assignment was chosen because the building he did his thesis on is similar to The Thames St. Wharf Office Building and located in Washington DC so it is relatively close geographically. Table 4.1 shows the results of the general conditions estimate.

The estimate is based on the ideal 26 month construction schedule. None of the delays are factored into the costs because their durations are unknown separate general. There is also no information available on who was responsible for the delays so it is unknown who is responsible for paying the extra general conditions costs. A separate estimate was compiled to show the general conditions costs per week to demonstrate the increased costs associated with any delay. Table 4.2 shows this estimate.

Each subcontractor is responsible for all of their own trailers, storage sheds and equipment. Additionally no cranes were provided by the construction managers. All on-site personnel are based on Armada Hoffler Construction's staffing. A list of additional assumptions is listed below.

Assumptions:

- 1. All months consist of four (4) weeks
- 2. Each subcontractor is responsible for their respective permits
- 3. Each subcontractor is responsible for their own safety equipment
- 4. Each subcontractor is responsible for clean up at the end of each work day
- 5. An average of four (4) dumpsters were pulled a week over the duration of the project
- 6. Each dumpster has the same cost associated per pull
- 7. All project staff are on-site for the entire project duration
- 8. Six (6) portable toilets are needed per month over the entire project duration.

General Conditions								
	Project Management Personel							
Item	Quantity	Amount	Units	Unit Price	Total			
PM	1	104	wk	\$ 1,925.00	\$ 200,200.00			
APM	1	104	wk	\$ 1,650.00	\$ 171,600.00			
PE	1	104	wk	\$ 1,350.00	\$ 140,400.00			
Superintendent	1	104	wk	\$ 2,025.00	\$ 210,600.00			
Assisstant Super	1	104	wk	\$ 1,600.00	\$ 166,400.00			
PMA	1	104	wk	\$ 1,200.00	\$ 124,800.00			
Safety Manager	1	104	wk	\$ 1,350.00	\$ 140,400.00			
				Subtotal	\$ 1,154,400.00			
	Adminis	strative Facilities	& Supplies					
Item	Quantity	Amount	Units	Unit Price	Total			
50x10 Trailer rent/month	2	26	Mo	\$ 275.94	\$ 14,348.88			
Office Eqpt Rental	1	26	Mo	\$ 152.21	\$ 3,957.46			
Office Supplies Avg	1	26	Mo	\$ 83.47	\$ 2,170.22			
Telephone	1	26	Mo	\$ 78.56	\$ 2,042.56			
lights & HVAC	1	26	Mo	\$ 147.30	\$ 3,829.80			
IT Expenses	1	26	Mo	\$ 75.00	\$ 1,950.00			
Drawings & Specifications	1	1	LS	\$ 5,000.00	\$ 5,000.00			
				Subtotal	\$ 33,298.92			
	J	obsite Requiren	nents					
Item	Quantity	Amount	Units	Unit Price	Total			
Fencing 6' High Chain Link	1	600	LF	\$ 8.73	\$ 5,238.00			
Dumpsters	4	26	Mo	\$ 200.00	\$ 20,800.00			
Temp Power	1	34625	CSF/Flr	\$ 2.65	\$ 91,756.25			
Temp Water	1	26	Mo	\$ 60.88	\$ 1,582.88			
Portable Toilets	6	26	Ea/month	\$ 179.21	\$ 27,956.76			
				Subtotal	\$ 147,333.89			
		Safety						
Item	Quantity	Amount	Units	Unit Price	Total			
Personal Protection Eqpt.	1	1	LS	\$ 1,000.00	\$ 1,000.00			
				Subtotal	\$ 1,000.00			
		Job Extras						
Item	Quantity	Amount	Units	Unit Price	Total			
Commissioning	1	\$ 50,000,000	Project	0.75%	\$ 375,000.00			
Concrete Testing	1	1	Project	\$ 35,378.00	\$ 35,378.00			
	•		-	Subtotal	\$ 410,378.00			
					\$ 1,746,410.81			

Table 4.1

General Conditions Cost Per Week								
Project Management Personel								
Item	Quantity	Amount	Units	Uni	t Price	Tot	al	
PM	1	1	wk	\$	1,925.00	\$	1,925.00	
APM	1	1	wk	\$	1,650.00	\$	1,650.00	
PE	1	1	wk	\$	1,350.00	\$	1,350.00	
Superintendent	1	1	wk	\$	2,025.00	\$	2,025.00	
Assisstant Super	1	1	wk	\$	1,600.00	\$	1,600.00	
PMA	1	1	wk	\$	1,200.00	\$	1,200.00	
Safety Manager	1	1	wk	\$	1,350.00	\$	1,350.00	
					Subtotal	\$	11,100.00	
	Administrative Facilities & Supplies							
Item	Quantity	Amount	Units	Uni	t Price	Tot	al	
50x10 Trailer rent/month	2	0.25	Мо	\$	275.94	\$	137.97	
Office Eqpt Rental	1	0.25	Мо	\$	152.21	\$	38.05	
Office Supplies Avg	1	0.25	Мо	\$	83.47	\$	20.87	
Telephone	1	0.25	Мо	\$	78.56	\$	19.64	
lights & HVAC	1	0.25	Мо	\$	147.30	\$	36.83	
IT Expenses	1	0.25	Мо	\$	75.00	\$	18.75	
				Sub	total	\$	272.11	
		Jobsite Require	ments					
Item	Quantity	Amount	Units	Uni	t Price	Tot	al	
Dumpsters	4	0.25	Мо	\$	200.00	\$	200.00	
Temp Water	1	0.25	Мо	\$	60.88	\$	15.22	
Portable Toilets	6	0.25	Ea/month	\$	179.21	\$	268.82	
	-			Sub	total	\$	484.04	
				Tot	al/ Week	\$	11,856.14	

Table 4.2

Critical Industry Issues

The 2009 PACE Roundtable was a day dedicated to presenting critical industry issues and topics and bringing them up in a way where they could be openly discussed between students, faculty and industry professionals. The theme of the roundtable was "Creating Opportunities." It was dedicated to how the downturn in the economy has affected the construction industry, what industry members and their companies are doing to remain profitable and where they see the construction industry going in the near future due to the economy. An industry panel on the "state of construction", a student panel on the "communication patterns of the now generation" and three breakout sessions each on a specific technical topic were used to address the changes in the industry. The three breakout topics covered were "Energy and the Construction Industry", "Business and Networking" and "BIM Executive Planning."

During the industry panel it became very apparent that during these slow economic times companies find themselves needing to diversify their projects. Companies are no longer able to stay in just one or two niche markets and survive. They are finding that other companies that they don't normally compete with are breaking into their markets and "steeling" their jobs out from under them. All of the panel members said that their companies need to branch out into new areas of construction that they have either not been a part of before or have had very little experience in. If they don't successfully do that they will have a lot of trouble in the coming months getting work and keeping employees busy. They are also finding that projects that used to have three or four companies bidding them now have 20-30 companies bidding. This creates a more cut throat market and it also increases the need for quality relationships between the construction contractors and the building owners. Another effect of so many companies bidding is that some companies low ball their bid values just to get the work and then deliver an inferior product because they aren't making as much as is desired.

Companies also seem to be using the downturn, and the slowing in the amount of work they are doing, to better themselves. They see increased time available to hold employee training sessions and to improve their project staffs. They have been able to focus more energy on improving the basics of the company and figure out where exactly as a company they are strongest and in what areas they need improvement.

The industry panel also briefly addressed BIM and what affect it is having on the industry and where they see it going in the future. Currently BIM is slowly emerging as a viable cost and time saving technique. It is difficult to gage how much time and money is actually saved using BIM at this point but it is known that companies that have used BIM on a project before and know that is being implemented on the project they are pursuing will submit a lower bid price. The panel members saw BIM become more widely used in the future but noted that the economy has halted some growth because people aren't as willing to spend the extra upfront money necessary implement BIM when they aren't exactly sure what it's returns are and the construction industry is naturally a slow evolving one. As firms and the people in them become more technologically savvy and more familiar with the programs BIM will become a strong force in the construction industry.

I attended the Business and Marketing breakout session and it was mostly geared towards finding ways to enter into new markets, improving relationships with clients, subcontractors and designers and different types of project delivery systems and communication lines. A fair amount of the issues that have occurred on The Thames St. Wharf project have been due to communication issues between multiple different parties and I felt that finding a way to address that would be a good thesis topic for me to look into. One thing that surprised me most about the discussion that took place was how much the industry members shared ideas with each other and explained the pros and cons of different ideas. Each member was openly sharing information with people they consider their competitors. It seemed very unusual that everyone was as open as they were with each other but it was a mutually beneficial atmosphere. Every member appeared to walk out of the room with a bit of knowledge that they did not have before.

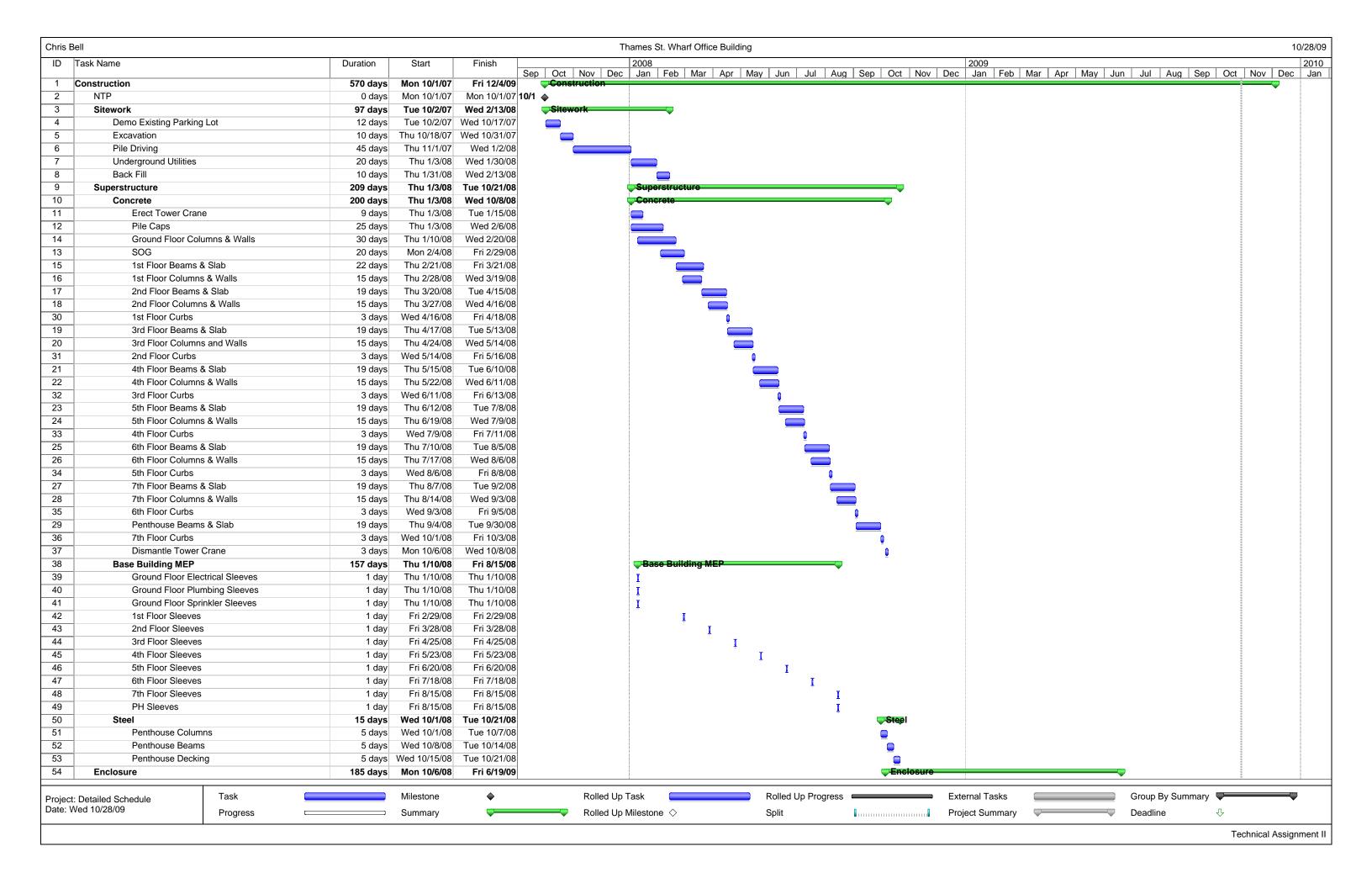
The biggest topic discussed in the breakout session, and something I may look into as a thesis research topic, was the Integrated Project Delivery method (IPD). On a project using the IPD the owner, architect and contractor sign a joint, incentives based contract. Both the contractor and architect are brought on in the very early stages of schematic design and work together in much the same way as they would with a Design-Build contract. The major difference with the IPD is that the owner is not cut off from the design and construction of the building. They are an integral member of the team and help direct the designers towards what they want. In a design-build relationship the owner is generally cut out from the design process once a designer is selected and told the basics of what the owner wants. I feel that if everyone was involved in the design process and had responsibilities through the end of construction that a lot of the coordination issues that occurred and something the expensive systems that needed to be installed in the building could have been avoided.

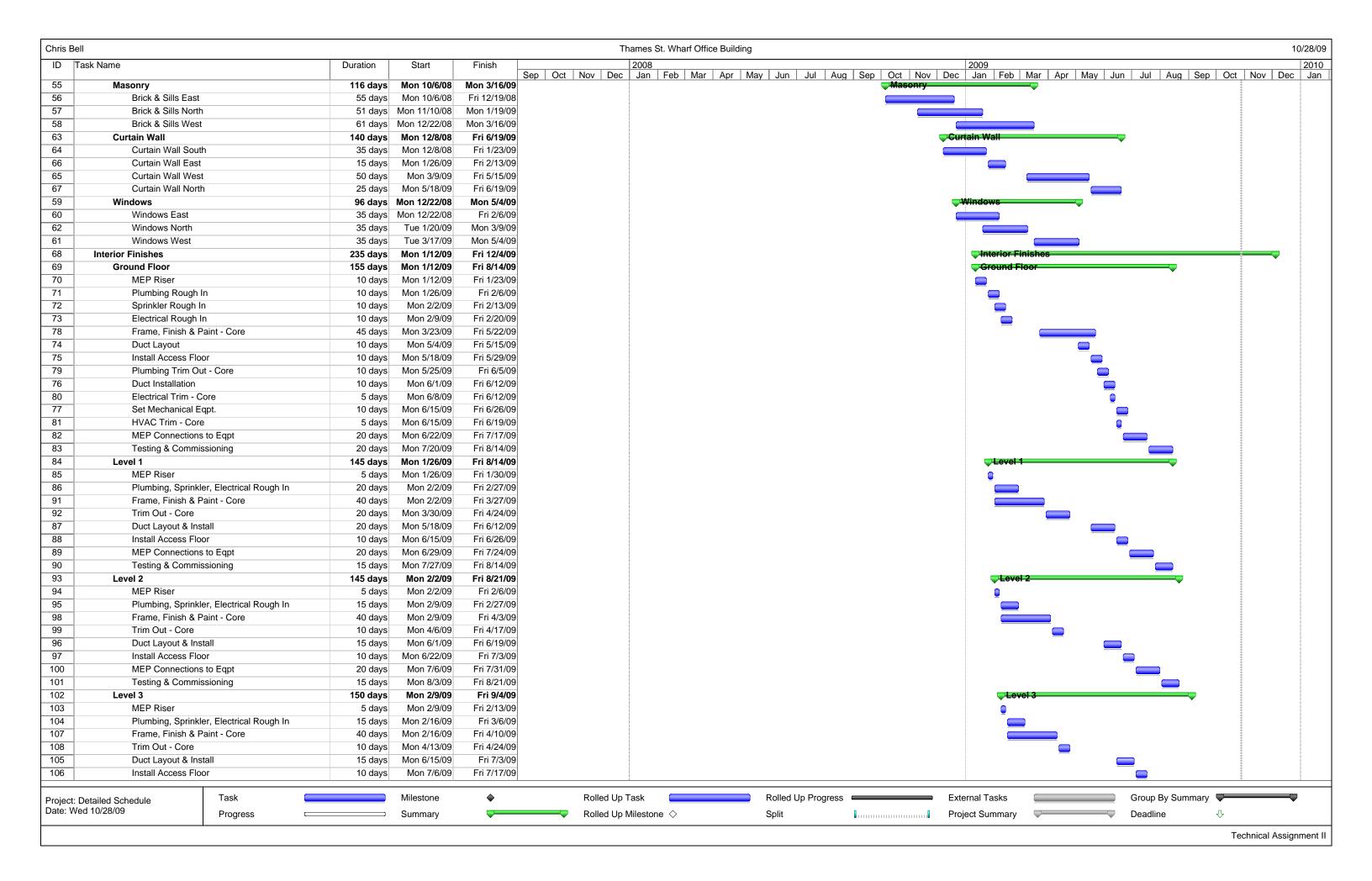
Another topic that was discussed during the breakout session that I may look into as a research topic instead of an IPD was using two CM or GC firms together as joint venture to deliver a project. If the Thames St. Wharf project was pursued as a joint venture between Struever Bros. and another firm it is possible that SBER may not have defaulted on their responsibilities preventing all of the issues that came along with that. Additionally the expertise of both parties could have possibly combined to prevent some of the other issues that have occurred.

All of the industry members I met and talked to were more than willing to help with anything that I may need in the future on my thesis but three really stood out as people that could be a great help. The first is Ray Sowers of ONCORE Construction. ONCORE was the concrete subcontractor on the Thames St. Wharf Office Building and he knows a lot of details about the project during the time that ONCORE was on-site. The second contact is Rob Leicht from DPR construction. DPR does a lot of IPD projects around the country and he may have some valuable insight into the delivery method or be able to put me in the direction of someone else who does. And the third contact is Mike Abbondante from Barton Malow. He has worked on at least two joint venture projects for Barton Malow and will be able to explain their benefits and inner workings in more detail.

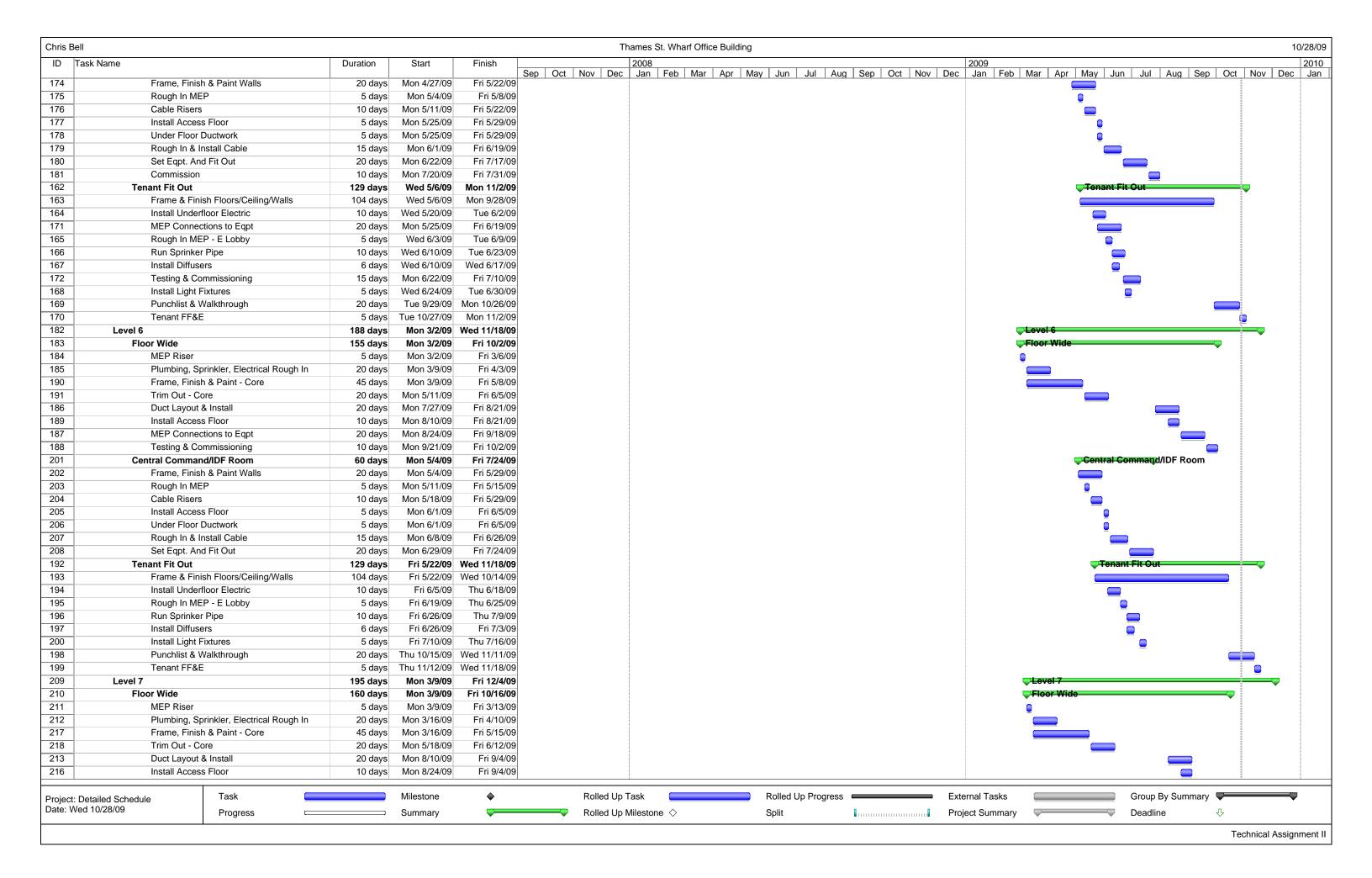
Appendix A

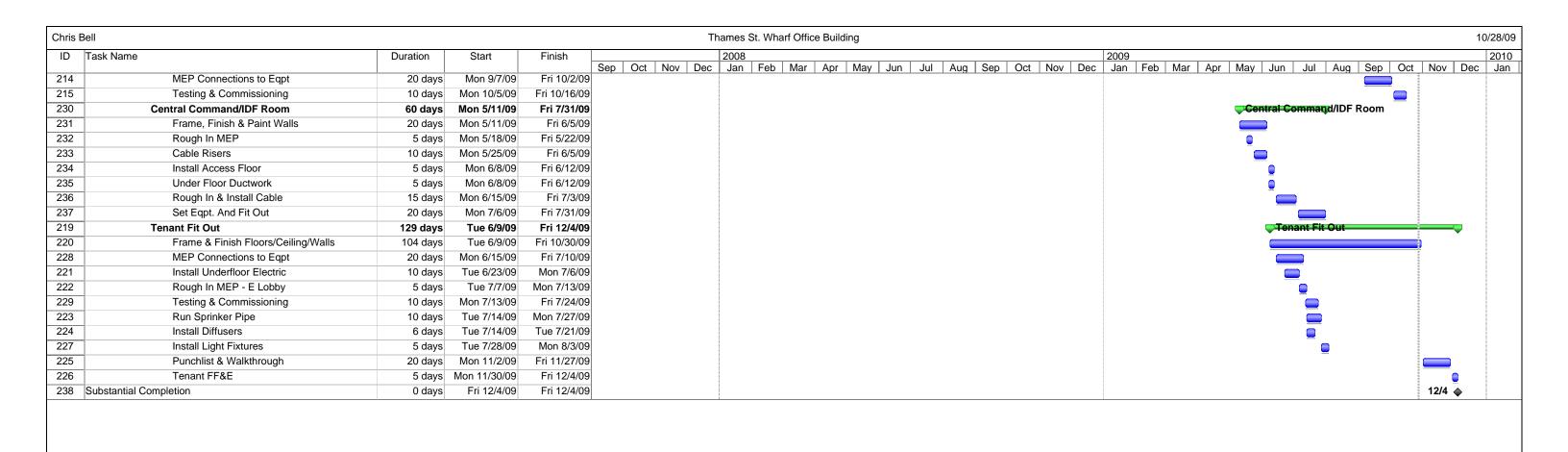
Detailed Project Schedule











Appendix B

Detailed Structural Estimate Overview

Piles							
Туре	Length[ft]	Diam[in]	Quantity	Total Length[ft]			
Standard Piles	60.46	14.00	262.00	15839.21			
Marine Piles	60.46	16.00	65.00	3929.58			

Concrete						
Superstructure						
Area	CY Per Floor	CY Total				
Elevated	575	4600				
Beams	537	4294				
Columns	75	597				
Shear Walls	98	780				
Curbs	25	203				
Total/Floor	1309					
Superstructure Total		10474				
Area		CY				
SOG		1342				
Pile Caps		456				
Shear wall Foundations		758				
Grade/Edge Beams		113				
4000 psi piles		627				
5000 psi piles		203				
		2=22				
Foundation Total		3500				
Building Total		13974				

Reinforcing				
	Mild			
Supe	rstructure			
Area	Tons Per Floor	Total Tons		
Elevated	6.81	54.44		
Beams	102.89	823.09		
Columns	7.75	62.03		
Shear Walls	6.62	53.00		
Curbs	1.79	14.31		
Total/Floor	125.86			
Superstructure Total		1007.36		

Foundation				
Area		Tons		
SOG		148.15		
Pile Caps & S.W.				
Foundations		99.21		
Grade/Edge Beams		5.26		
Piles		68.04		
Foundation Total		320.66		
Total Mild Reinforcing		1328.02		

Post Tension							
Area	Ft/ Floor	Ft Total	lbs/Ft	lbs			
Elevated	1450.00	11600.00	0.67	7772.00			
Beams	5663.74	45309.90	0.67	30357.63			
Total PT Cable				38129.63			